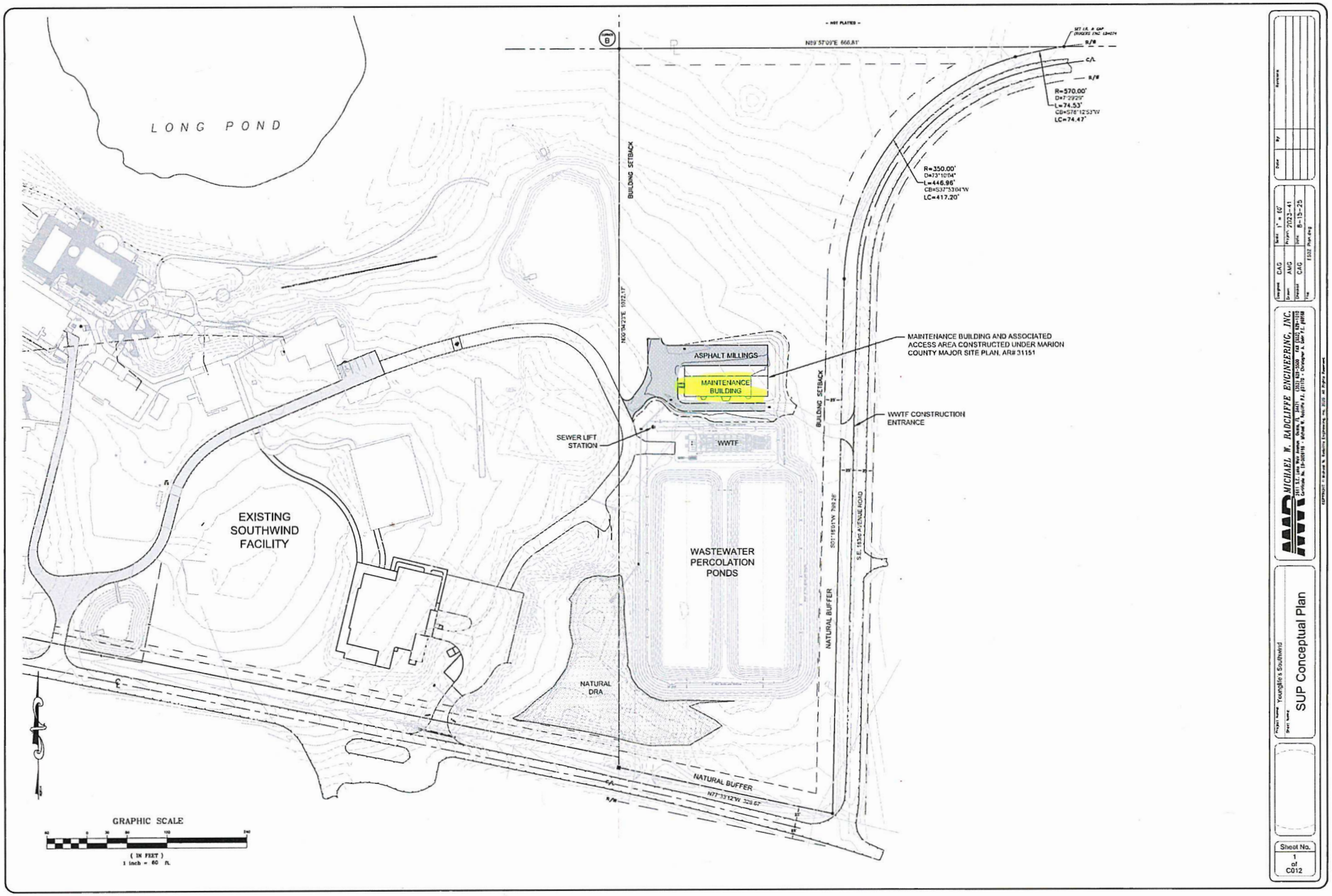
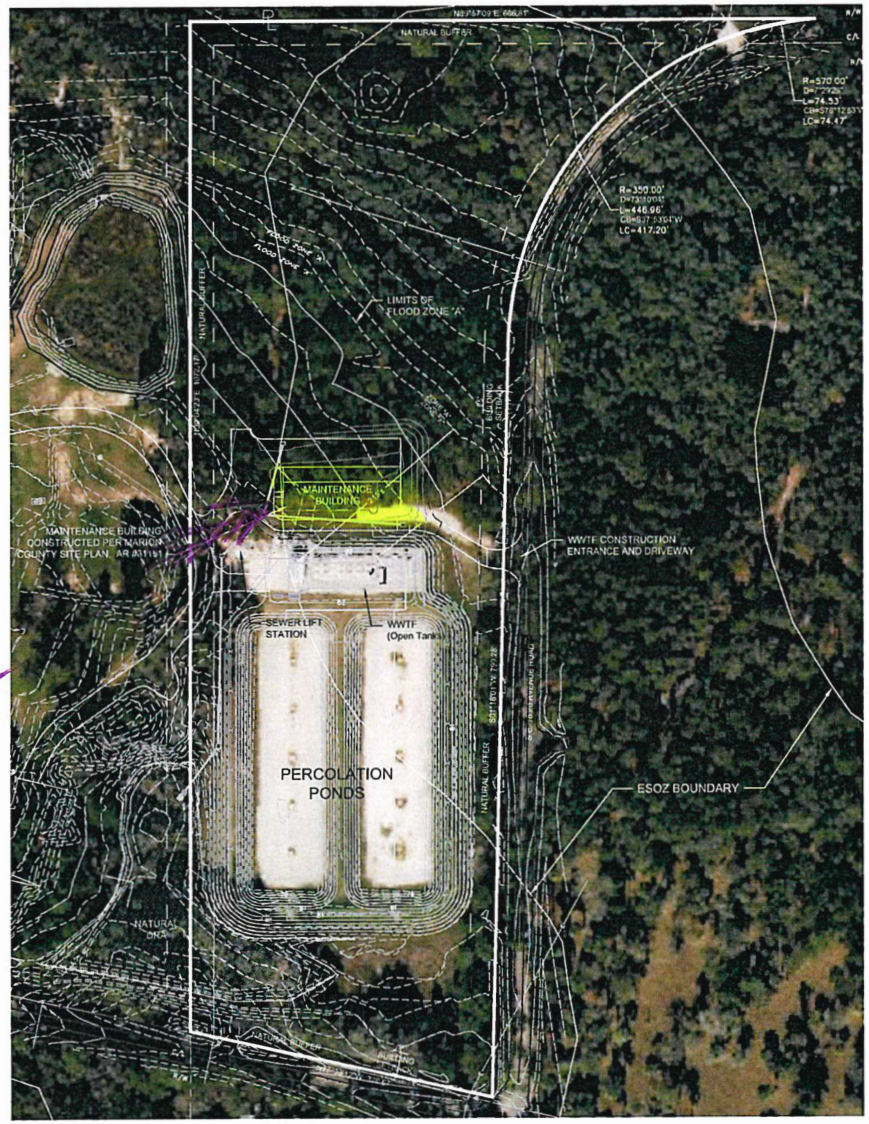

LONG POND
EXISTING SOUTHWIND FACILITY
BUILDING SETBACK
N00°04'23"E 1072.17'
N89°57'09"E 660.81'
ASPHALT MILLINGS
MAINTENANCE BUILDING
SEWER LIFT STATION
WWTF
WASTEWATER PERCOLATION PONDS
NATURAL DRA
NATURAL BUFFER
N77°33'12"W 323.67'
S01°18'01"W 799.28'
S.E. 193rd AVENUE ROAD
R=570.00'
Δ=7°29'29"
L=74.53'
CB=S78°12'53"W
LC=74.47'
R=350.00'
Δ=73°10'04"
L=446.96'
CB=S37°53'01"W
LC=417.20'
MAINTENANCE BUILDING AND ASSOCIATED ACCESS AREA CONSTRUCTED UNDER MARION COUNTY MAJOR SITE PLAN, AR# 31151
WWTF CONSTRUCTION ENTRANCE
GRAPHIC SCALE
(IN FEET)
1 inch = 60 ft.
MICHAEL W. RADCLIFFE ENGINEERING, INC.
Project Name Youngs's Southwind
Sheet Name SUP Conceptual Plan
Sheet No. 1 of C012

GRAPHIC SCALE

(IN FEET)
1 inch = 60 ft.

Younglife's Southwind Camp
ESOZ Plan



NOTES:

1. THIS PLAN PREPARED IN SUPPORT OF A SPECIAL USE PERMIT ON PARCEL#40263-001-00
2. BOUNDARY INFO FROM SURVEY BY ROGERS ENGINEERING, LLC DATED JUNE 30, 2020
3. THIS SITE IS IN THE VERY UPPER REACH OF THE ESOZ, AND OVERLAND FLOW DISTANCE TO THE OCKLAWAHA RIVER IS APPROXIMATELY TWO MILES.

ESOZ DRAINAGE CALCULATIONS

EXISTING IMPERVIOUS AREAS	
WWTF (COVERED PORTION)	1787 SF
WWTF CONTROL BLDG	312 SF
MAINTENANCE BUILDING	3750 SF
ASPHALT (PAVT & MILLINGS)	10541 SF
CONCRETE	200 SF
TOTAL	16590 SF

NOTES:

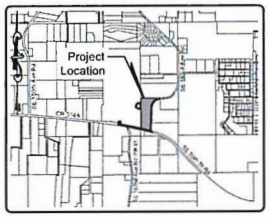
1. WWTF PERCOLATION PONDS REMOVED 65,000 SF OF AREA FROM PRE-DEVELOPMENT STORMWATER RUNOFF FROM THIS SITE. PERC PONDS ARE DESIGNED TO MEET FDEP VOLUME REQUIREMENTS AND PROVIDE 2 FEET OF FREEBOARD.
2. RUNOFF FROM THE MAINTENANCE BUILDING AND ASSOCIATED ASPHALT MILLINGS IS TREATED IN THE ADJACENT NATURAL RETENTION AREA, AS PERMITTED BY MARION COUNTY SITE PLAN, AR#31151.

TREES:

TREES REQUIRED = 9.07 X 100"/ACRE = 907 INCHES

TREES PROVIDED = BASED ON SURVEY LOCATED TREES, THERE ARE 1,238 INCHES PER ACRE.

THE NORTHEN 4 ACRES WILL REMAIN IN NATURAL STATES WITH A YIELD OF 4 x 1,238 = 4,952 INCHES. ADDITIONALLY, TREES WILL REMAIN IN THE 25' WIDE NATURAL BUFFER SHOWN ON THE PLAN.



LOCATION MAP
SCALE: 1" = 2000'
MARION COUNTY, FLORIDA
SECTION 19, TOWNSHIP 16 SOUTH, RANGE 25 EAST

Site Information:

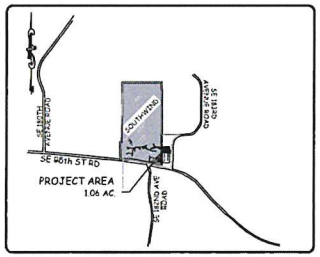
PROJECT NAME:	YOUNGLIFE'S SOUTHWIND SUP
LOCATION:	18285 SE 95TH STREET RD OCKLAWAHA, FL, 32179
OWNER:	YOUNG LIFE
CONTACT:	CHRIS GWIN, P.E. 2611 SE LAKE WEIR AVENUE OCALA, FLORIDA 34471
SITE AREA:	9.07 ACRES
EXISTING ZONING:	A-1
PARCEL NUMBER:	40263-001-00

MICHAEL W. RADCLIFFE ENGINEERING, INC.

Designed CAG
Drawn AMG
Checked CAG
Scale 1" = 60'
Project 2023-41
Date 8-15-25
File ESOZ Plan.dwg

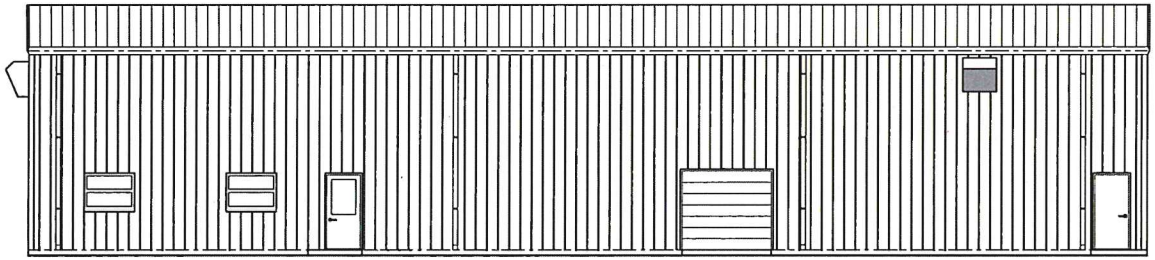
Project Name Younglife's Southwind Camp
Sheet Name ESOZ Plan

Sheet No. C001 of



LOCATION MAP

SCALE: 1" = 2000'
MARION COUNTY, FLORIDA
SECTION 19, TOWNSHIP 16 SOUTH, RANGE 25 EAST



Younglife Southwind Camp Maintenance Building

Site Information:

PROJECT NAME: YOUNG LIFE – SOUTHWIND

PROJECT LOCATION: 18115 SE 95th St Rd
OCKLAWAHA, FL 32179

PARCEL NUMBERS: 40287-000-00 AND 40263-001-00

PHASE ONE	AREA (Sq. Ft.)
Main Building & Workshops A & B	1,911
Main Building - Future Office Area	1,089
Lean-To	1,600
Total Phase One	4,600

Index of Sheets

1.0	Cover Sheet
2.0	Construction Notes
3.0	Site Layout Plan
4.0	Life Safety Plan
5.0	Floor Plan
6.0	Floor Plan Details
7.0	Foundation Plan
8.0	Elevations Plan
E-100	Electrical Plan
F-100	Future Floor Plan

MATERIAL LEGEND

PLYWOOD
TEXTURE 1-11
GYPSUM BOARD
FINISH WOOD
ROUGH WOOD
METAL
GLASS
BRICK
CONCRETE
CONCRETE BLOCK
COMPACTED EARTH
GRANULAR FILL
BATT INSULATION
RIGID INSULATION

List of Abbreviations

A.I.C.	- ALT. INTERRUPTING CAPACITY	L.L.H.	- LONG LEG HORIZONTAL
A.B.	- ANCHOR BOLT	L.L.V.	- LONG LEG VERTICAL
A.F.F.	- ABOVE FINISHED FLOOR	L.P.	- LOW POINT
ALT.	- ALTERNATE	LAV.	- LAVATORY
ALUM.	- ALUMINUM	LOC.	- LOCATE
AVG.	- AVERAGE	LOVD	- LOUVER
BD.	- BOARD	MAS.	- MASONRY
B.O.	- BOTTOM OF	MAX.	- MAXIMUM
BLK'G.	- BLOCKING	MEG.	- MEGNESIUM
BM.	- BEAM	MTD.	- MOUNTED
BOT. BOTT.	- BOTTOM	MTL.	- METAL
BRG.	- BRIDGE	N.I.C.	- NOT IN CONTRACT
℄	- CENTER LINE	O.C.	- ON CENTER
C. (CND)	- CONDUIT	O.D.	- OUTSIDE DIMENSION
CLG.	- CEILING	O.H.	- OVERHEAD
COL.	- COLUMN	O.U.O.	- UNLESS OTHERWISE
CONC.	- CONCRETE		
CONN.	- CONNECTION	P.	- PHASE (CLAMPER)
CONST.	- CONSTRUCTION	PL.	- PLATE
CTR.	- CENTER	PLYWD. PLYWD.	- PLYWOOD
C.W.	- COLD WATER	POS.	- POINT OF SALE
		PRESTRESS.	- PRESTRESSED
D.	- DIAMETER PHASE	P.S.F.	- POUNDS PER SQ. FOOT
DIA.	- DIAMETER DIAGRAM	P.S.I.	- POUNDS PER SQ. INCH
DBL.	- DOUBLE	PT.	- PARKING TREATED
DPR.	- DROP THRU	PTN.	- PARTITION
DWG.	- DRAWING	P.V.C.	- POLYVINYL CHLORIDE
E.F.	- EXHAUST FAN	R.D.	- ROOF DRAIN
EA.	- EACH	REINST.	- REINSTALL
ELECT.	- ELECTRIC	REINF'D	- REINFORCED
ELEV.	- ELEVATION	R.W.L.	- RAIN WATER LEADER
EQ.	- EQUAL		
EQUIP.	- EQUIPMENT	SQ.	- SQUARE
E.W.	- EACH WAY	S.S. (S.S.)	- STAINLESS STEEL
EXT.	- EXTERIOR	STL.	- STEEL
		STRUC.	- STRUCTURAL
F.C.O.	- FLOOR CLEAN OUT	SUSP. ACTS.	- SUSPENDED ACOUSTICS
F.D.	- FLOOR DRAIN	S.W.	- SAFE WASTE
F.E.	- FIRE EXTINGUISHER		
FIN. FL.	- FINISHED FLOOR	TAG	- TONGUE AND GROOVE
F.O.M.	- FACE OF MASONRY	T.O.S.	- TOP OF SLAB
F.O.S.	- FACE OF STUD	TYP.	- TYPICAL
F.R.P.	- FIBRE RATED PANELING		
F.S.	- FLOOR SINK	U.R.	- URINAL
FT.	- FOOT		
FTG.	- FOOTING	VEN.	- VENEER
		VERT.	- VERTICAL
GA.	- GAUGE	VEST.	- VESTIBULE
GALV.	- GALVANIZED	V.T.R.	- VENT THRU ROOF
G.C.	- GENERAL CONTRACTOR		
G.F.M.	- GLASS FIBER MOBAFE	W.	- WIDE WASTE
GRD.	- GROUND	W.C.	- WATER CLOSET
G.W.	- GREASY WASTE	W.C.	- WALL CLEANOUT
GYP.	- GYPSUM	WD.	- WOOD
		W.H.	- WATER HEATER
HORIZ.	- HORIZONTAL	W.P.	- WALK THROUGH
HT.	- HEIGHT	W.R.	- WATER PROOF
H.W.	- HOT WATER	W.W.P.	- WELDED WIRE FABRIC
H.D.	- HAND DRYER	W.W.M.	- WELDED WIRE MESH
H.P.	- HORSE POWER, HIGH POINT		
I.D.	- INTERIOR DESIGN		
INSUL.	- INSULATION		

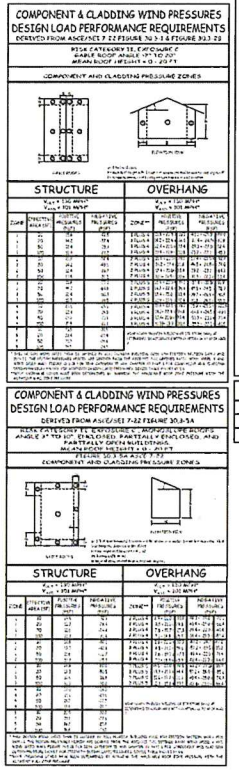
TYPE OF CONSTRUCTION

FBC CONSTRUCTION TYPE III-B (SECTION 602.3, TABLE 601)

USE AND OCCUPANCY CLASSIFICATION

MIXD USE: GROUP B BUSINESS/GROUP F-1 MODERATE HAZARD/STORAGE GROUP S-1 MODERATE HAZARD

GENERAL BLDG HEIGHT & AREA LIMITS (NONSPRINKLED)							OCCUPANT LOAD		OCCUPANT LOAD	
GROUP CLASSIFICATION	MAXIMUM HEIGHT	MAXIMUM STORIES	MAX. AREA PER STORY	PROVIDED HEIGHT	PROVIDED STORIES	PROVIDED AREA PER STORY	2023 FBC TABLE 1004.5 OCCUPANT LOAD FACTOR	2023 FFPC TABLE 7.3.1.2 OCCUPANT LOAD FACTOR	FBC	FFPC
FACTORY GROUP F-1 MODERATE HAZARD	55 FT	2	12,000 SF	18'-0" EAVE HT. MAX.	1 STORY	2,052 SF	100 GROSS	100 GROSS	20	20
STORAGE GROUP S-1 MODERATE HAZARD	55 FT	2	17,500 SF	18'-0" EAVE HT. MAX.	1 STORY	1,600 SF	300 GROSS	300 GROSS	5	5
BUSINESS GROUP B	55 FT	3	19,000 SF	18'-0" EAVE HT. MAX.	1 STORY	948 SF	150 GROSS	100 GROSS	6	9
								TOTAL OCCUPANT LOAD	31	34


COMPONENT & CLADDING WIND PRESSURES
DESIGN LOAD PERFORMANCE REQUIREMENTS
DERIVED FROM ASCE/SEI 7-22 FIGURE 30.3-2B

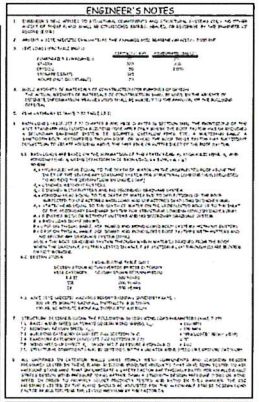
COMPONENT AND CLADDING PRESSURE ZONES

STRUCTURE | OVERHANG

COMPONENT & CLADDING WIND PRESSURES
DESIGN LOAD PERFORMANCE REQUIREMENTS
DERIVED FROM ASCE/SEI 7-22 FIGURE 30.3-3A

COMPONENT AND CLADDING PRESSURE ZONES

STRUCTURE | OVERHANG


ENGINEER'S NOTES

GARAGE DOORS WIND PRESSURES (PSF) EXPOSURE C
SEE FBC TABLE 1609.7(1) AND 1609.7(2)

CURINGTON Construction LLC
CGC 1531120
Office (352) 732-7939 Fax (352) 351-5044
2602 N.E. 24th Street Ocala, Florida 34470

H.W. Barrineau & Associates, Inc.
Civil Structural Engineering

FOR CONSTRUCTION

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CURINGTON CONTRACTING
ALL RIGHTS RESERVED
DRAWING # 24-5618

DATE:	4-25-24
REV:	12-10-24
REV:	12-21-24
REV:	12-23-24
REV:	12-26-24

Building Classification: Pre-Engineered Metal

Location: Parcel I.D. # 40263-001-00

Cover Page

Youngife Southwind Camp Maintenance Building

Drawn By: Pre-Engineered Metal
Checked By:
Date Completed:

Scale: 3/16"=1'-0" On 22" X 34" Paper
1/32"=1'-0" On 11" x 17" Paper

SHEET # 1.0

12/28/2024 5:08:31 AM

GENERAL REQUIREMENTS:

ANY REFERENCE TO CONTRACTOR SHALL ALSO INCLUDE ALL SUB CONTRACTORS AS THEY RELATE TO ANY AND ALL NOTES THROUGHOUT THESE PLANS

1. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS AND OTHER PROJECT DRAWINGS BY OTHER DISCIPLINES ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE CODES LISTED BELOW.
2. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS RELATING TO EXISTING CONDITIONS BY MAKING FIELD SURVEYS AND MEASUREMENTS PRIOR TO COMMENCING FABRICATION OR CONSTRUCTION
3. THE CONTRACTOR SHALL ENSURE THAT ALL CONSTRUCTION METHODS USED WILL NOT CAUSE DAMAGE TO ADJACENT BUILDINGS, UTILITIES, OR OTHER PROPERTY THIS REQUIREMENT IS PARTICULARLY IMPORTANT DURING FOUNDATION INSTALLATION

CONSTRUCTION RESPONSIBILITY:

THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETED STRUCTURE, AND ARE NOT INTENDED TO INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, SEQUENCES, AND FOR JOB SAFETY. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENT.

SUBMITTALS:

1. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE CONTRACTOR PRIOR TO SUBMITTAL. SUBMITTAL WITHOUT CONTRACTOR REVIEW RESULTS IN DELAYS. THE CONTRACTOR SHALL CONFIRM THAT SHOP DRAWINGS HAVE BEEN COMPLETED AND CHECKED BY THE SUPPLIER PRIOR TO SUBMISSION.
2. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS: (IF APPLICABLE)
   - STRUCTURAL STEEL
   - STRUCTURAL STEEL CONNECTIONS (a,c)
   - STEEL JOISTS (b)
   - PRE-ENGINEERED TRUSSES AT ROOF GABLES (a,c)
   - REINFORCING STEEL-THAT IS ABNORMAL OR IF NOT SHOWN ON PLAN
   - FORMWORK, SHORING, RESHORING (a,c)
   - CONCRETE MIX DESIGNS-THAT ARE ABNORMAL OR WITH STRENGTH GREATER THAN 3500 PSI
   - ELEVATORS (c)
3. THE NOTATIONS FOLLOWING SUBMITTAL ITEMS INDICATE THE FOLLOWING:
   a. CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.
   b. INCLUDE A CERTIFICATE OF COMPLIANCE WITH CONTRACT
   c. DOCUMENTS SIGNED AND SEALED BY THE PROFESSIONAL
   d. ENGINEER REGISTERED IN THE STATE OF FLORIDA RESPONSIBLE
   e. FOR THE DESIGN.
   f. SUBMIT FOR INFORMATION AND RECORD ONLY.
4. MANUFACTURER'S LITERATURE: SUBMIT 4 COPIES OF MANUFACTURER'S LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.

SPECIFICATIONS

1. GENERAL BUILDING CODE.
   A. 2023 FLORIDA BUILDING CODE, 8TH EDITION
2. CONCRETE CODES.
   A. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-14). SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301).
   B. LATEST EDITION OF THE CRSI MANUAL OF STANDARD PRACTICE, AND ALL SUPPLEMENTS.
3. STRUCTURAL STEEL CODES: (IF APPLICABLE)
   A. AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, LOAD AND RESISTANCE FACTOR DESIGN, AISC 360-16.
   B. AISC MANUAL OF STEEL CONSTRUCTION, 2017.
4. PRECAST AND PRECAST-PRESTRESSED REQUIREMENTS. SEE SPEC'S BY SUPPLIER (IF APPLICABLE)
5. OPEN WEB STEEL JOISTS AND JOIST GIRDERS: (IF APPLICABLE)
   A. STANDARD SPECIFICATIONS, LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS, SJI-17.
6. MASONRY CONSTRUCTION.
   A. ACI 530-13/ASCE 5-13 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.
   B. ACI 530.1-13/ASCE 6-13 SPECIFICATIONS FOR MASONRY STRUCTURES
7. STEEL DECKS: (IF APPLICABLE)
   A. AISI S100-16 NORTH AMERICAN SPECIFICATIONS FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
   B. SDI-C 2017 STANDARD FOR COMPOSITE STEEL FLOOR DECK SLABS.
   C. ANSI/SDI D-10-10 STANDARD FOR STEEL ROOF DECK.
8. 2023 FLORIDA ACCESSIBILITY CODE.
9. AISC STEEL CONSTRUCTION MANUAL, 2017.
10. AISC 360-16 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS".
11. AISC STEEL CONSTRUCTION MANUAL, 2017, "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS".
12. AMERICAN WELDING SOCIETY (AWS) D1.1:D1.1M: 2020 "STRUCTURAL WELDING CODE - STEEL".
13. ASTM A-6/A-6M-14 "STANDARD SPECIFICATION FOR GENERAL REQUIREMENTS FOR ROLLED STRUCTURAL STEEL BARS, PLATES, SHAPES AND SHEET"
14. ANSI/ASHRAE/62-01 "VENTILATION FOR ACCEPTABLE INDOOR AIR QUALITY."

PRIMARY CODES :

8TH EDITION, 2023 FLORIDA BUILDING CODE

8TH EDITION, 2023 FLORIDA PLUMBING CODE

8TH EDITION, 2023 FLORIDA MECHANICAL CODE

8TH EDITION, 2023 FLORIDA FUEL GAS CODE

8TH EDITION, 2023 FLORIDA ENERGY CONSERVATION CODE

OTHER REFERENCED CODES:

"FLORIDA FIRE PREVENTION CODE-7th EDITION"

NFPA: NATIONAL FIRE PROTECTION ASSOCIATION

NEC: NATIONAL ELECTRICAL CODE 2020

LOCAL ENVIRONMENTAL REGULATIONS
ACI 318 BUILDING REQUIREMENTS FOR REINFORCED CONCRETE.
OTHER STANDARD LISTED IN THEIR RESPECTIVE SECTIONS IN SPECIFICATIONS

CAST-IN-PLACE CONCRETE:

1. THE LATEST EDITION OF THE FOLLOWING ACI STANDARDS APPLY.
   ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
   ACI 216.1 CODE REQUIREMENTS FOR DETERMINING FIRE RESISTANCE OF CONCRETE AND MASONRY CONSTRUCTION ASSEMBLIES
   ACI 360-10 GUIDE TO DESIGN OF SLABS-ON-GROUND.
2. CONCRETE SHALL BE NORMAL WEIGHT (150 PCF) AS SPECIFIED ON STRUCTURAL DRAWINGS, WITH MIXES DESIGNED TO MEET THE FOLLOWING CRITERIA FOR USE IN VARIOUS ELEMENTS OF THE STRUCTURE AS FOLLOWS.

STRUCTURAL ELEMENT	28 DAY COMPRESSIVE STRENGTH (PSI)		MAX SIZE AGGREGATE	MAX W/C RATIO
GRADE BEAMS, FOUNDATION WALLS	3000	1"	0.48	
SLAB-ON-GRADE, ELEVATED FLOORS, BEAMS, COLUMNS	3000	3/4"	0.48	

3. SLUMP RANGE AT POINT OF DISCHARGE 4" TO 6".
4. WATER REDUCING AND AIR ENTRAINING AGENTS SHALL BE INCLUDED IN DESIGN MIXES. SUPERPLASTICIZERS MAY BE USED AT THE CONTRACTORS OPTION
5. A CONCRETE MIX DESIGN FOR EACH UNIQUE COMBINATION OF STRENGTH, COARSE AGGREGATE GRADATION AND WATER CEMENT RATIO SPECIFIED SHALL BE PREPARED BY AN INDEPENDENT TESTING LABORATORY AND BE SUBMITTED FOR REVIEW PRIOR TO CASTING ANY CONCRETE. MIXES THAT WILL BE TRANSPORTED AT THE PROJECT SITE BY PUMPING SHALL BE SPECIFICALLY DESIGNED FOR PUMPING.
6. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

CONCRETE FORMWORK: (IF APPLICABLE)

1. ALL FORMWORK SHALL BE DESIGNED, ERECTED, SUPPORTED, BRACED, AND MAINTAINED ACCORDING TO ACI 347, RECOMMENDED STANDARD PRACTICE FOR CONCRETE FORMWORK.
2. RESPONSIBILITY. THE DESIGN, CONSTRUCTION, AND SAFETY OF ALL FORMWORK SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALL FORMS, SHORES, BACKSHORES, FALSEWORK, BRACING, AND OTHER TEMPORARY SUPPORTS SHALL BE ENGINEERED TO SUPPORT ALL LOADS IMPOSED INCLUDING THE WET WEIGHT OF CONCRETE, CONSTRUCTION EQUIPMENT, LIVE LOADS, LATERAL LOADS DUE TO WIND AND WET CONCRETE IMBALANCE. SEE SPECIFICATIONS FOR DETAILED REQUIREMENTS.
3. DESIGN. THE DESIGN OF ALL CONCRETE FORMWORK SHALL BE CERTIFIED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA. SEE SPECIFICATIONS
4. TOLERANCES. UNLESS SPECIFIED OTHERWISE, ALL TOLERANCES FOR FORMWORK SHALL CONFORM TO ACI STANDARD 117, STANDARD TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS. THE CONTRACTOR SHALL ENGAGE A LICENSED SURVEYOR TO VERIFY THAT WORK IS WITHIN SPECIFIED TOLERANCES UNLESS WRITTEN AUTHORIZATION IS OBTAINED FROM THE ENGINEER TO PROVIDE TOLERANCE CONTROL USING THE CONTRACTOR'S OWN FORCES PRIOR TO BEGINNING WORK.
5. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED OR TOOLED WHERE SHOWN ON THE ARCHITECTURAL DRAWINGS.

ROUGH FRAMING: (IF APPLICABLE)

1. INSTALL MAIN FRAMING MISCELLANEOUS BLOCKING , FURRING , NAILING STRIPS, FRAMING AND SHEATHING. INSTALL MEMBERS TRUE, PLUMB AND LEVEL. INSTALL SHIMMING WHERE REQUIRED TO SET FRAMING IN PROPER ALIGNMENT. SECURE FRAMING IN PLACE. TRUSS AND SPACE MISCELLANEOUS FRAMING NO MORE THAN 24" ON CENTER. STUDS + FURRING TO BE NO MORE THAN 16" O.C. CONSTRUCT MEMBERS OF CONTINUOUS PIECES OF LONGEST POSSIBLE LENGTHS. PROVIDE ROUGH HARDWARE AS INDICATED.
2. MAKE PROPER PROVISIONS FOR WORK OF OTHER TRADES. REFER TO THE DRAWINGS FOR WOOD BLOCKING AND PLYWOOD REQUIRED AS BACK UP AND FRAMED OPENINGS FOR ALL OTHER AND THEIR ACCESSORIES.

SHEATHING AND FLOORING: (IF APPLICABLE)

1. PLACE PLYWOOD OR O.S.B. ROOF SHEATHING WITH REQUIRED JOINT SPACES BETWEEN SHEETS AND WITH JOINTS STAGGERED. PLYWOOD GRAIN SHALL BE PERPENDICULAR TO FRAMING . SECURE SHEETS OVER FIRM BEARING. PROVIDE PLYWOOD SHEATHING CLIPS(REFERRED TO AS "H" CLIPS OR PSC CLIPS) AT UNSUPPORTED PLYWOOD ROOF EDGES: SPACED ONE BETWEEN EACH SUPPORT. PROVIDE EDGE BLOCKING AT ALL ROOF OPENINGS. NAIL TO FRAMING MEMBER AT PLYWOOD EDGES AT 6" O.C. AND AT INTERMEDIATE SUPPORTS AT 6" O.C. NAIL WITH AT LEAST 80 RING-SHANK NAILS. (FOR ADDITIONAL INFORMATION SEE ROOF NAILING AND CEILING DIAPHRAGM, SHEATHING LAYOUT AND ROOF SHEATHING NAILING ZONES)

FABRICATION OF BEAMS AND BUILT UP HEADERS: (IF APPLICABLE)

1. AT HEADERS AND BEAMS BUILT UP WITH MULTIPLE 5/8" #1/#2 2X MEMBERS. NAIL TOGETHER WITH 10D NAILS @ 16" ON CENTER ALONG EACH EDGE AND WITH 2 16 NAIL PER 6" NORMAL DEPTH OF HEADER. PROVIDE PLYWOOD SPACER BETWEEN 2X MEMBERS TO WIDEN HEADER TO THE WIDTH OF THE STUD WALL FRAMED HEADERS ABOVE WALL OPENINGS SHALL BE BUILT USING THE FOLLOWING SIZE LUMBER

BUILT UP MEMBER LOCATION	2X6 FRAMING	2X4 FRAMING
BUILT UP HEADERS	3-2X8	2-2X10
BUILT UP BEAMS	3-2X12	3(.25"X 10.5") MS

2. ALL NON LOAD BEARING HEADERS LESS THAN 3' SHALL BE 3-2X4, ALL NON LOAD BEARING HEADERS GREATER THAN 3' SHALL BE 2 2X4

REINFORCING STEEL:

1. REINFORCING STEEL ASTM A615/A615M-12. SPECIFICATION FOR DEFORMED AND PLAIN BILLET STEEL BARS FOR CONCRETE REINFORCEMENT, GRADE 60
2. REINFORCING STEEL TO BE WELDED ASTM A706/A706M-09b SPECIFICATION FOR LOW-ALLOY STEEL DEFORMED AND PLAIN BARS FOR CONCRETE REINFORCEMENT (FOR FABRIC ANCHORS, ETC)
3. WELDED WIRE FABRIC ASTM A185/A185M-14a CARBON STEEL WIRE AND WELDED WIRE REINFORCEMENT, PLAIN AND DEFORMED. FOR CONCRETE MINIMUM YIELD STRENGTH OF 65 KSI (W1.2 AND LARGER IN SIZE) & 70 KSI (LESS THAN W1.2 IN SIZE)
4. REINFORCING STEEL COVER
   A. CONCRETE CAST DIRECTLY AGAINST EARTH 3"
   B. INTERIOR SLABS 1"
   C. INTERIOR BEAMS AND COLUMNS 1-1/2"
   D. EXTERIOR BEAMS AND COLUMNS 2"
   E. EXTERIOR SLABS 1-1/2"
5. WHERE SPLICE LENGTHS ARE NOT SPECIFIED, USE 40 BAR DIAMETERS BUT IN NO CASE LESS THE 24" LAP AT SPLICE.
6. SCHEDULED OR DETAILED REINFORCING STEEL SHALL NOT BE TACK WELDED FOR ANY REASON. WELDED REINFORCING STEEL SPLICES ARE NOT PERMITTED.
7. LAP ALL WELDED WIRE FABRIC A MINIMUM DISTANCE OF ONE CROSS WIRE SPACING PLUS 2 INCHES.
8. ALL REINFORCING STEEL SHALL BE SUPPORTED ON STANDARD STOOLS, HELD RIGIDLY AND ACCURATELY IN PLACE, AND PROTECTED AGAINST DISPLACEMENT BEFORE AND DURING PLACEMENT OF CONCRETE. SUPPORTING AGGRESSIVE LEGS THAT REST ON CONCRETE SURFACES THAT WILL BE EXPOSED IN THE FINISHED STRUCTURE SHALL BE FABRICATED OF STAINLESS STEEL OR PLASTIC COATED
9. FOUNDATION AND GRADE BEAM REINFORCING SHALL BE SUPPORTED ON SPECIALLY CAST 3 INCH HIGH CONCRETE BLOCKS CAST IN ACCORDANCE WITH DETAILS FURNISHED ON DRAWINGS. SLAB-ON-GRADE REINFORCING, INCLUDING WIRE FABRIC, SHALL BE SUPPORTED ON ACCESSORIES WITH SAND PLATES OR ON PRECAST BLOCKS ON CONCRETE BRICK OF THE PROPER THICKNESS.

FOUNDATIONS:

1. FOUNDATION DESIGN IS BASED ON PRESUMPTIVE LOAD BEARING INFORMATION CONTAINED IN SECTION 1806 OF THE LATEST EDITION OF THE FLORIDA BUILDING CODE AND ENGINEERING FROM STEELCOR BUILDINGS, INC DIGITALLY SIGNED AND SEALED PLANS DATED 19 APRIL 2021.
2. ALL VEGETATION, TOPSOILS, ROOTS AND ORGANIC ZONES SHALL BE STRIPPED AND REMOVED FROM THE CONSTRUCTION AREA FOR A DISTANCE OF AT LEAST 10 FEET BEYOND THE EXTERIOR OF BUILDING FOUNDATION LIMITS. THE DEPTH OF STRIPPING SHALL BE THAT REQUIRED TO REMOVE SIGNIFICANT ROOT ZONES, SMALL TREE STUMPS AND OTHER UNACCEPTABLE MATERIAL, BUT IN NO CASE LESS THAN 6 INCHES
3. EXCAVATION FOR LARGE STUMPS, ABANDONED UTILITIES, UNDERGROUND TANKS, ETC. SHALL BE BACK FILLED IN LAYERS WITH COMPACTION AND TESTING OF EACH LAYER AS DESCRIBED FOR PLACEMENT AND COMPACTION OF FILL MATERIAL. USE LOOSE BACK FILL LAYER THICKNESS APPROPRIATE FOR THE SIZE OF COMPACTOR BEING USED.
4. THE UPPER 12 INCHES OF THE FOOTING EXCAVATION BOTTOMS SHOULD BE COMPACTED TO DENSITIES EQUIVALENT TO 95 PERCENT OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY, (ASTM D-698 METHOD C). COMPACTION OR RECOMPACTIONS OF THE FOOTING EXCAVATION BEARING LEVEL SOILS LOOSENED BY THE EXCAVATION PROCESS, SHALL BE ACHIEVED BY MAKING SEVERAL PASSES WITH A RELATIVELY LIGHTWEIGHT WALK-BEHIND VIBRATORY SLED OR ROLLER COMPACTOR. TEST ONE OUT OF EVERY SIX COLUMN FOOTINGS AND ONE TEST EVERY 150 LINEAL FEET OF WALL FOOTING
5. UNLESS NOTED, ALL INTERIOR FOOTINGS SHALL BE CENTERED UNDER COLUMNS, PIERS AND WALLS.
6. SLAB-ON-GRADE CONSTRUCTION SHALL BE SUPPORTED ON SUBGRADE AND TO A DENSITY OF NO LESS THAN 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D-1557 METHOD C) TO A DEPTH OF AT LEAST 12 INCHES. SLABS-ON-GRADE SHALL BE CAST OVER A VAPOR BARRIER. SEE SPECIFICATIONS.
7. SURFACE WATER RESULTING FROM RAINFALL DURING CONSTRUCTION WILL REQUIRE CONTROL. IN ADDITION, THE CONTRACTOR SHALL BE PREPARED TO PROVIDE GROUNDWATER CONTROL DURING CONSTRUCTION OF FOOTINGS, PITS, AND DURING UTILITY INSTALLATION. SUMPS AND PERIMETER TRENCHES SHALL BE LOCATED OUTSIDE SLAB AND FOOTING BEARING AREAS TO AVOID LOOSENING THE BEARING SOILS. THE GROUNDWATER LEVEL SHALL BE MAINTAINED AT LEAST ONE FOOT BELOW THE BOTTOM OF ANY EXCAVATIONS MADE DURING CONSTRUCTION AND TWO FEET BELOW THE SURFACE OF ANY VIBRATORY COMPACTION OPERATIONS.

CONCRETE MASONRY: (IF APPLICABLE)

1. CONCRETE MASONRY UNITS SHALL BE LOAD BEARING TYPE CONFORMING TO ASTM C-90, TYPE II, HAVING A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI (NET AREA)
2. MORTAR SHALL CONFORM TO ASTM C-270 TYPE S.
3. PLAIN END TWO CORED UNITS SHALL BE USED FOR BLOCKS THAT ARE TO HAVE CELLS REINFORCED AND FILLED. WEB SHELLS ADJACENT TO CELLS THAT ARE TO BE FILLED ARE TO BE BEVELED IN MORTAR
4. FILL CELLS AS NOTED ON DRAWINGS WITH 3000 PSI COARSE GROUT CONFORMING TO ASTM C-476 OR WITH 3000 PSI FEA-STONE CONCRETE MIX SPECIFICALLY DESIGNED FOR FILLING OF CELLS.
5. TIE BEAMS CAST ON MASONRY WALLS SHALL BE CONSTRUCTED SO AS TO KEY AND BOND INTO BLOCK CELLS. THE USE OF BUILDING PAPER OR SHEET PLASTIC TO CLOSE VOIDS BELOW ITS NOT ALLOWED. HOWEVER LENTEL PAPER IS ALLOWED
6. SEE ARCHITECT'S DRAWINGS FOR THE EXTENT AND EXACT LOCATION OF MASONRY WALLS.
7. MASONRY WALLS SHALL BE BRACED EITHER BY OTHER INTERSECTING WALLS OR STRUCTURAL MEMBERS OR BY ANCHORAGE OR BRACING TO THE STRUCTURE ABOVE
8. BLOCK LINTELS SHALL BE SPECIALLY FORMED U-SHAPED LINTEL WITH REINFORCING BARS, OR PRECAST UNITS DESIGNED FOR THE WEIGHT OF MASONRY ABOVE AND OTHER APPLIED LOADS. SEE DETAIL
9. ALL MASONRY WALLS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED EDGE VERTICAL AND LATERAL FORCES IN THE FINAL CONSTRUCTED CONFIGURATION ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ADEQUATELY BRACE THE WALLS FOR VERTICAL AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF LATERAL SUPPORT BY CONNECTIONS AT FLOORS OR ROOF FRAMING LEVELS.
10. AT ALL INTERIOR CMU WALLS PROVIDE 1 #5 VERTICAL REINFORCING AT THE FOLLOWING LOCATIONS (TIE INTO FOOTING AND TIE BEAM)
    A. ALL DOOR AND WINDOW JAMBS
    B. WALL INTERSECTIONS
    C. AT MAXIMUM OF 4' 0" O.C.
11. ALL VERTICAL REINFORCING BARS TO TIE TO FOOTING DOWELS AND BOND BEAM WITH STANDARD HOOK FOR FULL DEVELOPMENT OF EXTERIOR SHEARWALLS.

STRUCTURAL STEEL:

1. SEE NOTES ON PRIMARY CODES AND SPECIFICATIONS
2. MATERIALS:

STEEL SHAPES AND PLATES	ASTM A36
STEEL PIPE	ASTM A53, TYPE E OR S, GRADE B
STEEL TUBING	ASTM A500, GRADE B (F46)
HIGH STRENGTH BOLTS	ASTM A325
MACHINE BOLTS	ASTM A307
ANCHOR BOLTS	ASTM A307 OR A36 ROD
WELDING ELECTRODES	E70 XX SERIES

3. NON-SHRINK, NON-METALLIC GROUT WITH A 28 DAY STRENGTH OF 5000 PSI SHALL BE USED UNDER BASE PLATES AND SHALL CONFORM TO CORPS OF ENGINEERS CRD-621, FACTORY PREMIX GROUT.
4. TEMPORARY BRACING OF STRUCTURAL STEEL ELEMENTS IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. PROVIDE ONE SHOP COAT OF RED OXIDE PRIMER (TT-P-636) ON ALL STEEL EXCEPT FOR ITEMS TO BE HOT DIPPED GALVANIZED.
6. FRAMING CONNECTIONS NOT DETAILED SHALL BE DESIGNED FOR THE REACTION SHOWN ON THE PLAN. IF NO REACTION IS PROVIDED, DESIGN FOR 1/2 THE BEAM MAXIMUM UNIFORM LOAD PER PART 2 OF THE AISC MANUAL FOR STEEL CONSTRUCTION.
7. ALL WELD OPERATORS SHALL BE AWS QUALIFIED
8. SHOP CONNECTIONS SHALL BE WELDED. USE 1/4" FILLET WELD MINIMUM
9. FIELD CONNECTIONS SHALL BE WELDED AS DETAILED
10. SUBMIT FOR REVIEW SHOP DRAWINGS OF STEEL DETAILS PRIOR TO APPROVING DESIGN & INSTALLATION OF STRUCTURAL STEEL.
11. CONCRETE EXPANSION ANCHORS SHALL CONFORM TO GSA SPECIFICATION FF-S-325.
12. ALL ELEMENTS NOTED TO BE GALVANIZED SHALL BE HOT DIPPED. IN ACCORDANCE WITH ASTM A153 AFTER SANDBLAST CLEANING PER SSPC SP3. USE ASTM A325 BOLTS HOT DIPPED GALVANIZED WITH GALVANIZED HARDENED WASHERS AND GALVANIZED HEAVY HEX NUTS FOR BOLTING OF GALVANIZED ITEMS.

STEEL JOISTS AND JOIST GIRDERS: (IF APPLICABLE)

1. SEE NOTES ON PRIMARY CODES AND SPECIFICATIONS
2. THE MANUFACTURER OF THE JOISTS AND JOIST GIRDERS SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE
3. PROVIDE ONE SHOP COAT OF RED OXIDE PRIMER (TT-P-636)
4. REFER TO MANUFACTURER'S DETAILS FOR SPECIAL TREATMENT OF NON STANDARD DEPTH JOIST SEATS SUPPORTING CONCENTRATED LOADS.

   SLOPED JOIST SEATS:
   JOIST GIRDER BOTTOM CHORD CONNECTION AT BEARINGS.
   JOIST GIRDER BOTTOM CHORD LATERAL SUPPORT.
5. ROOF JOISTS AND JOIST GIRDERS SHALL BE DESIGNED TO RESIST A NET UPLIFT DETERMINED BY WIND LOAD CRITERIA
6. CONNECT JOIST BEARING PLATES TO JOIST TOP CHORD FOR OR.
7. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS
8. SUBMIT FOR REVIEW SHOP DRAWINGS OF JOIST DETAILS FOR FABRICATION AND ERECTION PRIOR TO FABRICATING JOISTS.
9. PROVIDE HORIZONTAL OR DIAGONAL TYPE BRIDGING FOR ALL JOISTS AS REQUIRED BY SJI SPECIFICATION. THE ENDS OF ALL BRIDGING LINES TERMINATING AT BEAMS SHALL BE ANCHORED THERETO AT TOP AND BOTTOM CHORDS. PROVIDE ALL REQUIRED BRIDGING ANCHORS.
10. SECOND FLOOR JOISTS IN BUILDING SUPPORTS LIGHTWEIGHT CMU PARTITIONS. JOISTS HAVE BEEN DESIGNED USING EQUIVALENT UNIFORM LOADING. MANUFACTURER SHALL VERIFY THAT 90,000# LOAD MAY BE PLACED AT ANY POINT ON JOIST TOP CHORD

STEEL ROOF DECK: (IF APPLICABLE)

1. ROOF DECK SHALL BE 1-1/2" DEEP. SEE ROOF SCHEDULE FOR GAGE AND PROFILE DESIGNATION.
2. ROOF DECK SHALL BE PLACED SO AS TO COVER AT LEAST TWO SPANS. NO SINGLE SPAN CONDITIONS SHALL BE USED.
3. STEEL DECK SHALL CONFORM TO ASTM A446 OF A GRADE TO PROVIDE A MINIMUM YIELD STRENGTH OF 33,000 PSI.
4. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO ASTM A525 G90 CLASS.
5. SEE ROOF PLAN AND DETAILS FOR ROOF DECK ATTACHMENT AND FORCES IMPOSED DUE TO UPLIFT AND DIAPHRAGM SHEAR UNDER WIND LOADING. SEE SPECIFICATIONS FOR INSPECTION AND REPORTING REQUIRED ON ROOF DECK ATTACHMENT.
6. PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS. END LAPS OF SHEET SHALL BE A MINIMUM OF TWO INCHES AND SHALL OCCUR OVER SUPPORTS

COLD-FORM STEEL SPECIFICATIONS

GENERAL. THE DESIGN OF COLD FORMED CARBON AND LOW ALLOY STEEL STRUCTURAL MEMBERS SHALL BE IN ACCORDANCE WITH AISI S100. THE DESIGN OF COLD-FORMED STAINLESS-STEEL STRUCTURAL MEMBERS SHALL BE IN ACCORDANCE WITH ASCE 8.

COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION SHALL ALSO COMPLY WITH SPECIFICATIONS WHERE REQUIRED. THE SEISMIC DESIGN OF COLD FORMED STEEL STRUCTURES SHALL BE IN ACCORDANCE WITH THE ADDITIONAL PROVISIONS OF THESE SPECIFICATIONS.

STEEL DECKS. THE DESIGN AND CONSTRUCTION OF COLD FORMED STEEL DECKS SHALL BE IN ACCORDANCE WITH THE FOLLOWING:

- NON COMPOSITE STEEL FLOOR DECKS. NON COMPOSITE STEEL FLOOR DECKS SHALL BE PERMITTED TO BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH ANSI/SDI NC1.0
- STEEL ROOF DECK. STEEL ROOF DECKS SHALL BE PERMITTED TO BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH ANSI/SDI-RD1.0
- COMPOSITE SLABS ON STEEL DECKS. COMPOSITE SLABS OF CONCRETE AND STEEL DECK SHALL BE PERMITTED TO BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH SDI-C

SEISMIC REQUIREMENTS FOR COLD FORMED STEEL STRUCTURES. WHERE A RESPONSE MODIFICATION COEFFICIENT, R, IS IN ACCORDANCE WITH ASCE 7, TABLE 12.2-1, IS USED FOR THE DESIGN OF COLD FORMED STEEL STRUCTURES. THE STRUCTURES SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THE REQUIREMENTS OF AISI S100, ASCE 8, OR, FOR COLD FORMED STEEL SPECIAL-BUILT MOMENT FRAMES, AISI S110.

TEST STANDARD FOR JOIST HANGERS AND CONNECTORS. ALLOWABLE LOADS FOR JOIST HANGERS AND CONNECTORS SHALL BE IN ACCORDANCE WITH EITHER AISI S914 OR THE PROCEDURE IN ASTM D1761. ALLOWABLE LOADS FOR HOLD-DOWNS SHALL BE IN ACCORDANCE WITH AISI S913

COLD-FORMED STEEL LIGHT FRAME CONSTRUCTION

GENERAL. THE DESIGN AND INSTALLATION OF STRUCTURAL AND NONSTRUCTURAL MEMBERS UTILIZED IN COLD FORMED STEEL LIGHT FRAME CONSTRUCTION WHERE THE SPECIFIED MINIMUM BASE STEEL THICKNESS IS NOT GREATER THAN 0.0180 INCHES (0.597 MM) SHALL BE IN ACCORDANCE WITH AISI S200 AND THESE SPECIFICATIONS, OR AISI S220 AS APPLICABLE.

HEADER DESIGN. HEADERS, INCLUDING BOX AND BACK TO BACK HEADERS, AND DOUBLE AND STRAIGHT HEADERS SHALL BE DESIGNED IN ACCORDANCE WITH AISI S210 OR AISI S100.

TRUSS DESIGN. COLD FORMED STEEL TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH AISI S214. THESE SPECIFICATIONS AND ACCEPTED ENGINEERING PRACTICE.

TRUSS DESIGN DRAWINGS. THE TRUSS DESIGN DRAWINGS SHALL CONFORM TO THE REQUIREMENTS OF SECTION B2.3 OF AISI S214 AND SHALL BE PROVIDED WITH THE SHIPMENT OF TRUSSES DELIVERED TO THE JOB SITE. THE TRUSS DESIGN DRAWINGS SHALL INCLUDE THE DETAILS OF PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING IN ACCORDANCE WITH SECTION B4(A) OR B 4(C) OF AISI S214 WHERE THESE METHODS ARE UTILIZED TO PROVIDE RESTRAINT/BRACING

DEFERRED SUBMITTALS.

DEFERRED SUBMITTALS ARE NOT ALLOWED.

TRUSSES SPANNING 60 FEET OR GREATER.

A REGISTERED DESIGN PROFESSIONAL SHALL BE USED FOR THE DESIGN OF THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING FOR TRUSSES WITH CLEAR SPANS 60 FEET (18.288 MM) OR GREATER.

STRUCTURAL WALL STUD DESIGN

STRUCTURAL WALL STUDS SHALL BE DESIGNED IN ACCORDANCE WITH EITHER AISI S211 OR AISI S100

FLOOR AND ROOF SYSTEM DESIGN

FRAMING FOR FLOOR AND ROOF SYSTEMS IN BUILDINGS SHALL BE DESIGNED IN ACCORDANCE WITH EITHER AISI S210 OR AISI S100

LATERAL DESIGN. LIGHT-FRAME SHEAR WALLS, DIAGONAL STRAP BRACING THAT IS PART OF A STRUCTURAL WALL AND DIAPHRAGMS USED

TO RESIST WIND, SEISMIC AND OTHER IN PLANE LATERAL LOADS SHALL BE DESIGNED IN ACCORDANCE WITH AISI S213.

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DRAWING # 24-5618

DATE:	4-24-24
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REV:	12-12-24
REV:	12

YOUNG LIFE SOUTHWIND
MAINTENANCE BUILDING

Grade Notes:

WHEN NEEDED TO HELP CLARIFY THE INTENT OF A PROPOSED GRADE, THE FOLLOWING NOTATIONS MAY BE USED:

A = TOP OF ASPHALT
C = TOP OF CONCRETE
G = TOP OF GROUND (SOD, MULCH, DIRT, ETC.)
F = FLUSH (ADJACENT SURFACES ARE FLUSH)
M = MATCH (MATCH EXISTING ELEVATION)

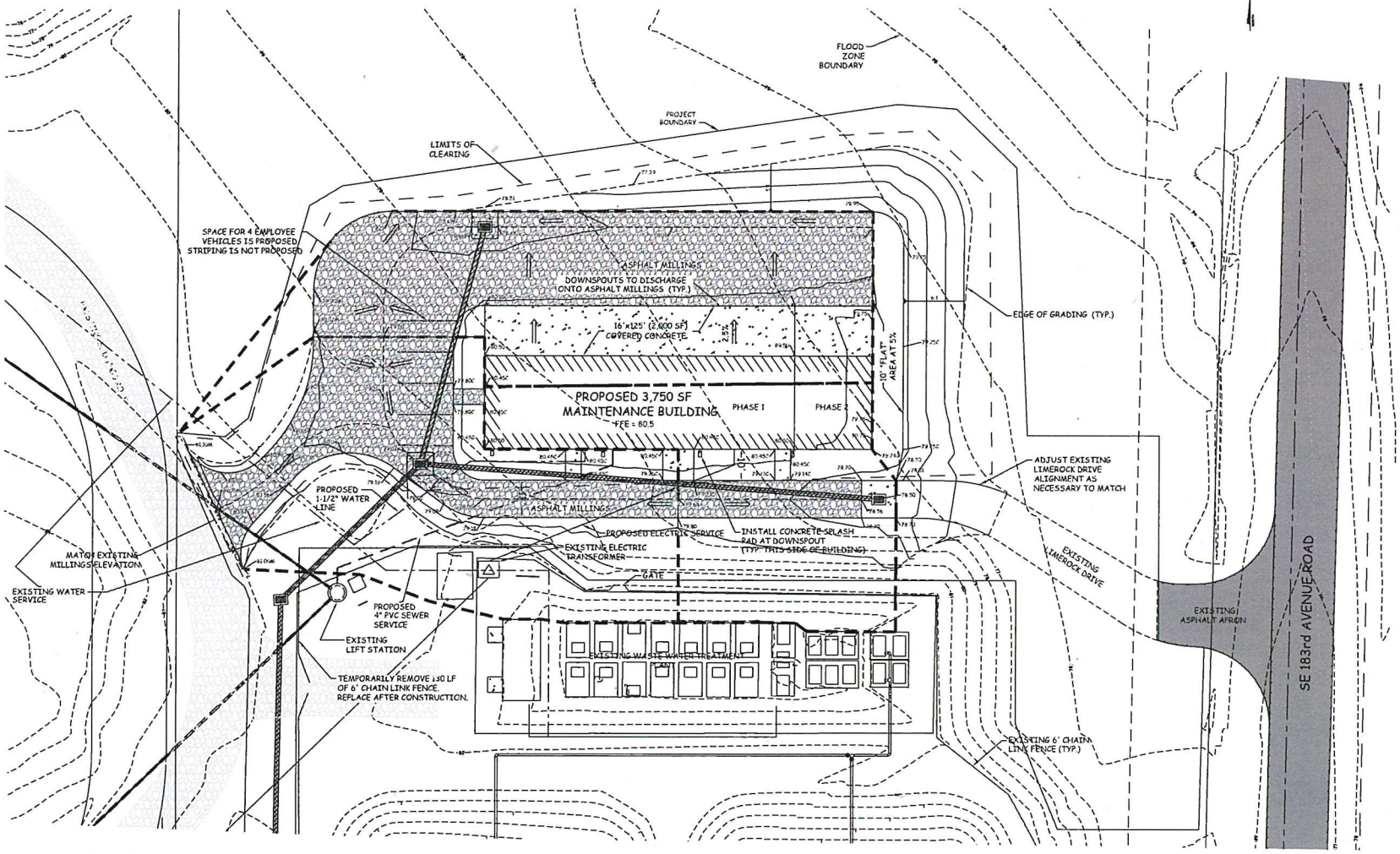
PROPOSED GRADES SHOWN AT THE JUNCTION OF PAVEMENT AND CONCRETE SHALL BE ASSUMED TO BE A PROPOSED PAVEMENT ELEVATION UNLESS OTHERWISE NOTED ABOVE.

Legend:

= FLOW ARROW
= PROPOSED GRADE
= GRADE BREAK LINE

PROPOSED MAINTENANCE BUILDING ESTIMATED QUANTITIES:

ASPHALT MILLINGS	= 1,111 SY
FILL	= 920 YDS (BANK)
15" HP STORM DRAIN PIPE	= 556 LF
TYPE C INLET STRUCTURES	= 5
CONC. COLLAR AT INLET STRUCTURES	= 3
CLEARING	= 0.54± ACRES
4" SEWER SERVICE LINE	= 104'
1" WATER SERVICE LINE	= 86'
SOD	= 992 SY



Site Layout Plan

Youngl.fe Southwind Camp Maintenance Building	Building Classification: Pre-Engineered Metal
Drawn By: Hector Rivera	Scale: Not To Scale
Checked By: Dan Curington	Location: Parcel I.D.# 40263-001-(4)

SHEET # 3.0

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DRAWING # 24-5618

DATE:	4-5-24
REV:	12-19-24
REV:	12-23-24
REV:	12-23-24
REV:	12-26-24

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H.W. Barrineau & Associates, Inc.
Civil & Structural Engineering

FOR CONSTRUCTION

12/28/2024 5:15:34 AM

LIFE SAFETY LEGEND

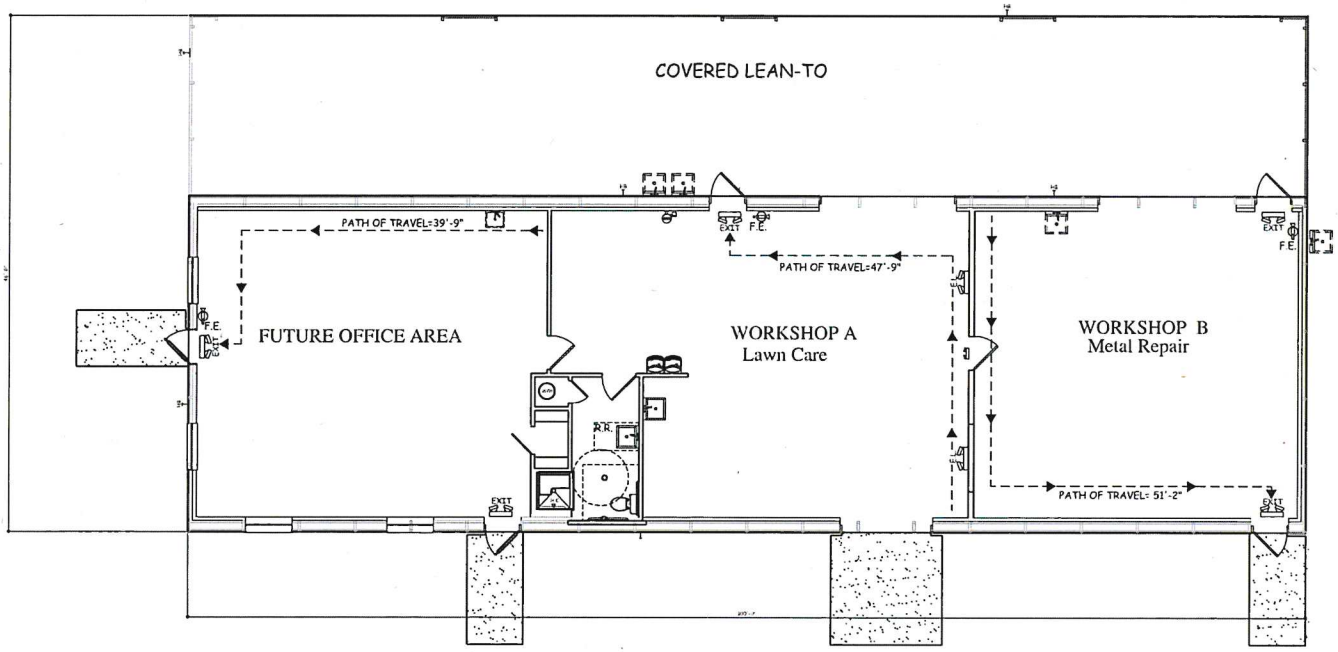
- MAX EXIT TRAVEL DISTANCE / PATH OF TRAVEL
- EXIT — EMERGENCY LIGHT /EXIT SIGN COMBO WITH BATTERY BACKUP
- EXIT — EXIT SIGN WITH BATTERY BACKUP
- E.L. — EMERGENCY LIGHT WITH BATTERY BACKUP
- F.E. — FIRE EXTINGUISHER
- F.S. — FIRE STROBE
- F.S. — FIRE PULL STATION
- S.D. — SMOKE DETECTOR

MEANS OF EGRESS

Means of Egress for Factory Group F-1, Storage Group S-1 and Business Group B per FBC Sections 1017 and FBC Table 1017.2 and per FFPC (NFPA 101 Life Safety Code 2021) Chapters 38, 40 & 42, Sections 38.2.6, 40.2.6 and 42.2.6.

The following is a summary:

Area	Common Path of Travel	Maximum Travel Distance
Group B & Group F-1	75 FT.	200 FT.(FBC) & 200 FT. (FFPC)
Group S-1	50 FT.	200 FT.(FBC) & 200 FT. (FFPC)



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FOR CONSTRUCTION

H.W. Barrineau & Associates, Inc.
Civil Structural Engineering

Life Safety Plan

Youngslife Southwind Camp
Maintenance Building
Drawn By: Luke Martin
Date Typical
Checked By: Don Curington

Building Classification: Pre-Engineered Metal
Location: Parcel I.D.# 40263-001-00
Scale: 1/16=1'-0" On 22" X 34" Paper 1/32=1'-0" On 11" x 17" Paper

SHEET # 4.0

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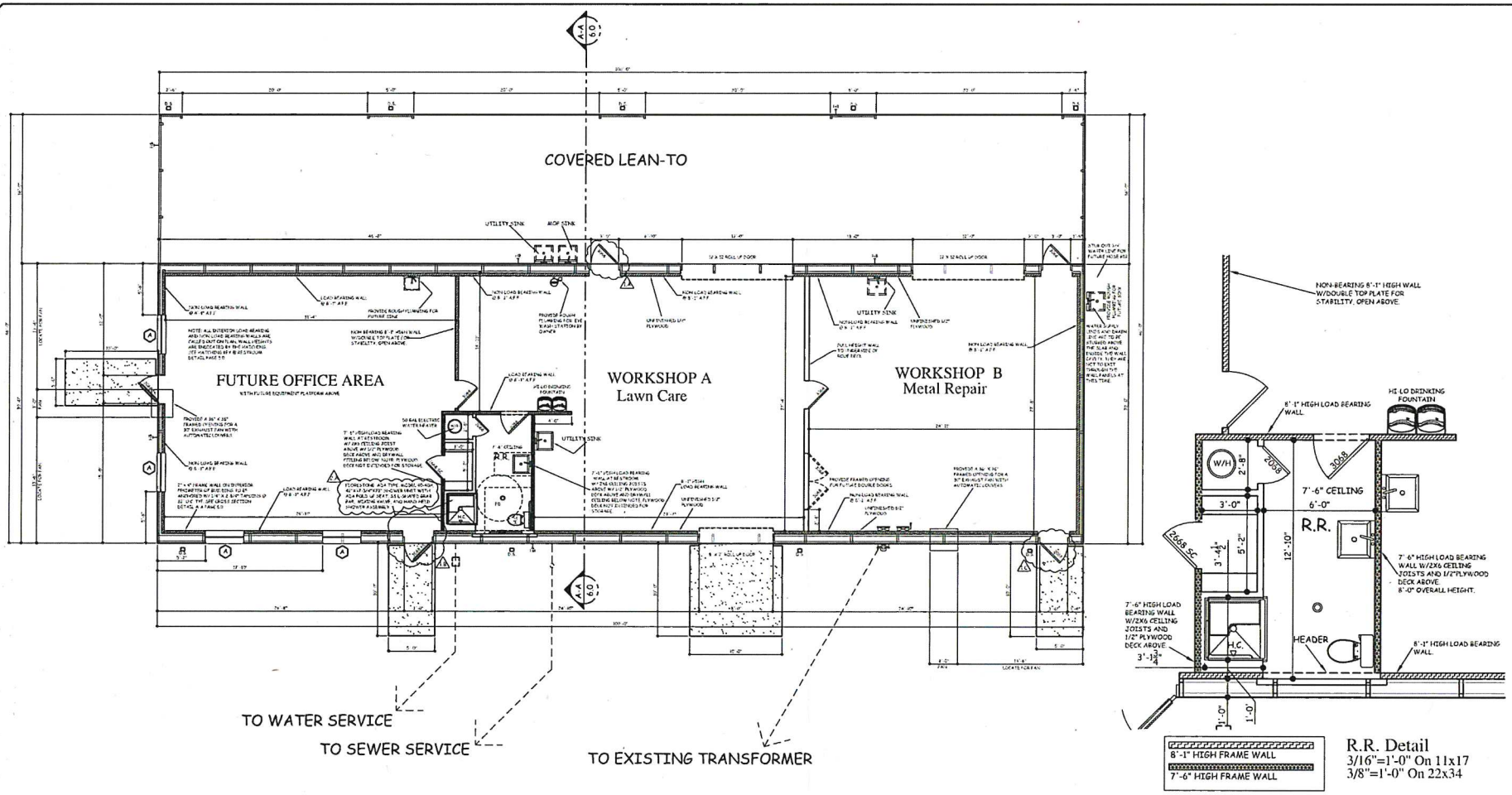

R.R. Detail
3/16"=1'-0" On 11x17
3/8"=1'-0" On 22x34

△ REVISIONS
1-A DOOR SWING REVERSED.
1-B DOOR SWING REVERSED.
1-C DOOR SWING REVERSED.
2-A. SHOWER MODEL CHANGE.

WINDOW SCHEDULE

CALLOUT	WINDOW SIZE	NOTES
A	4030	SINGLE HUNG FIN

FOR CONSTRUCTION

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H.W.Barrineau & Associates, Inc.
Civil & Structural Engineering

Building Classification: Pre-Engineered Metal
Youngslife Southwind Camp Maintenance Building
Location: Parcel I.D.# 40263-001-00
Drawn By: Luis Mayrical
Checked By: Dan Curington
Scale: 3/16"=1'-0" On 22" X 34" Paper 3/32"=1'-0" On 11" x 17" Paper

Floor Plan

SHEET # 5.0

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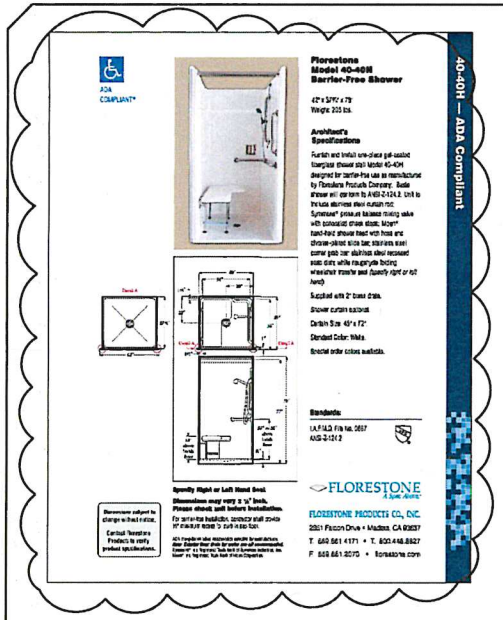
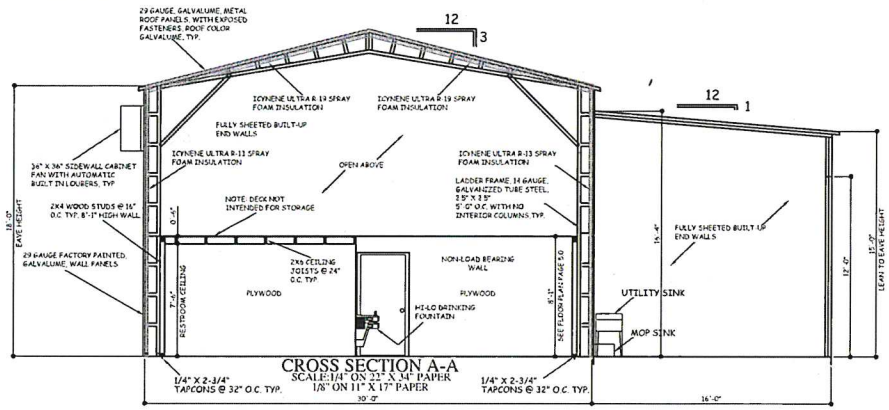
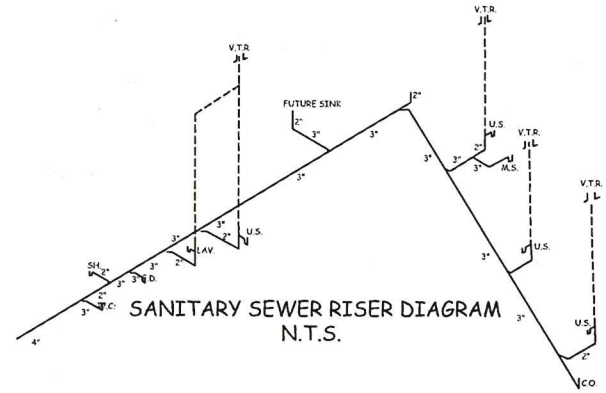
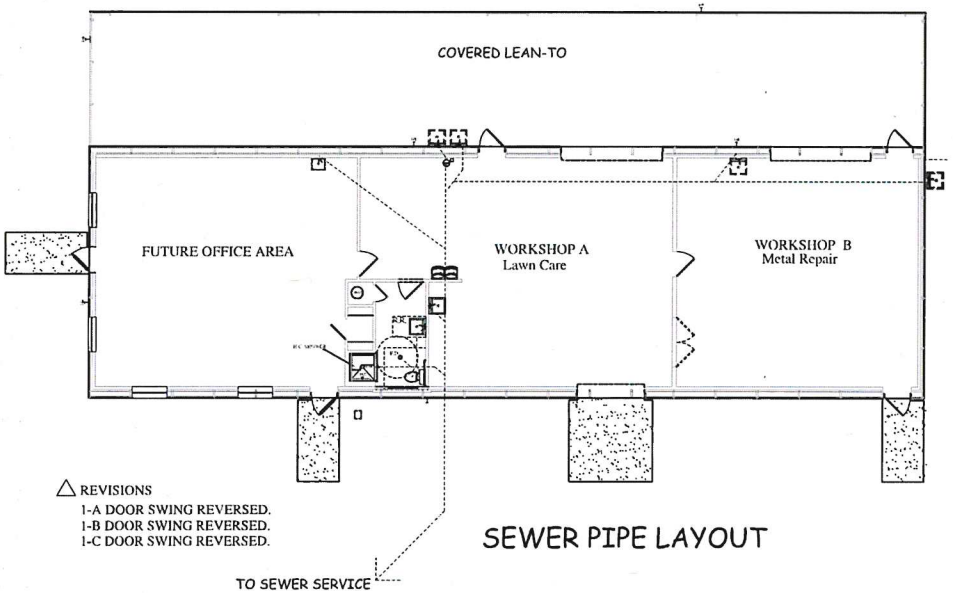




CROSS SECTION A-A
SCALE: 1/4" ON 22" X 34" PAPER
1/8" ON 11" X 17" PAPER

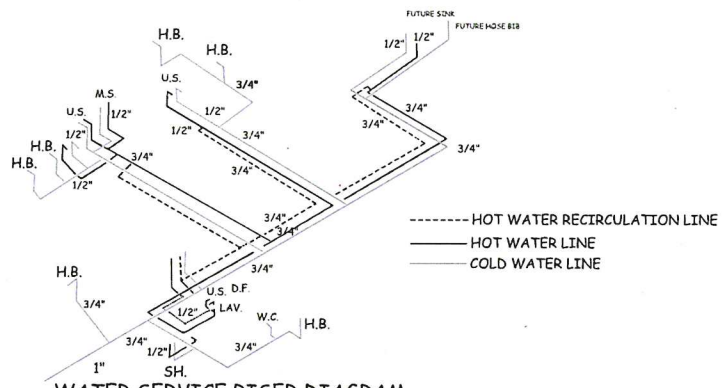


SANITARY SEWER RISER DIAGRAM
N.T.S.



△ REVISIONS
1-A DOOR SWING REVERSED.
1-B DOOR SWING REVERSED.
1-C DOOR SWING REVERSED.

SEWER PIPE LAYOUT



WATER SERVICE RISER DIAGRAM
N.T.S.

FOR CONSTRUCTION

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JOB NO. 24-5618

DATE:	4-25-24
REV:	12-10-24
REV:	12-23-24
REV:	12-23-24
REV:	12-26-24

CURINGTON Construction LLC
CGC 1511150
Office (352) 732-7839 Fax (352) 351-5044
2202 N.E. 24th Street Ocala, Florida 34470

H.W. Barrineau & Associates, Inc.
Civil Structural Engineering
Phone: (352) 629-1900 • AC: (352) 629-1902 Fax: (352) 629-1903 • Email: hwbarrineau@hwbeng.net
Mailing Address: 1 SE 1st Avenue, Ocala, Florida 34471-2320
Physical Address: 1 SE 1st Avenue, Ocala, Florida 34471-2320
Certificate of Authorization: Florida Licensed Engineering Business Number 26-0000 • Florida Licensed Architectural Business Number 26-0000

Floor Plan Details

Youngtife Southwind Camp
Maintenance Building

Building Classification:
Pre-Engineered Metal

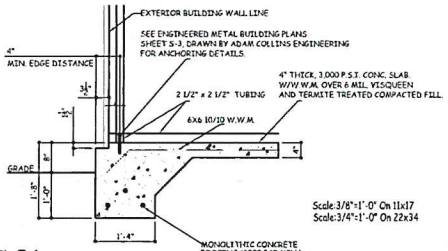
Location:
Parcel I.D.# 40263-001-00

Drawn By: Luis Martical
Checked By: Luis Martical
Date Certified: 12/23/24

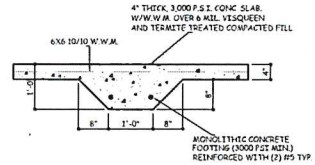
Scale:
3/16"=1'-0" On 22" X 34" Paper
3/32"=1'-0" On 11" X 17" Paper

SHEET #
6.0

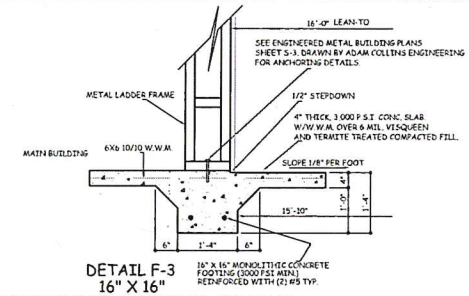
12/28/2024 5:18:11 AM



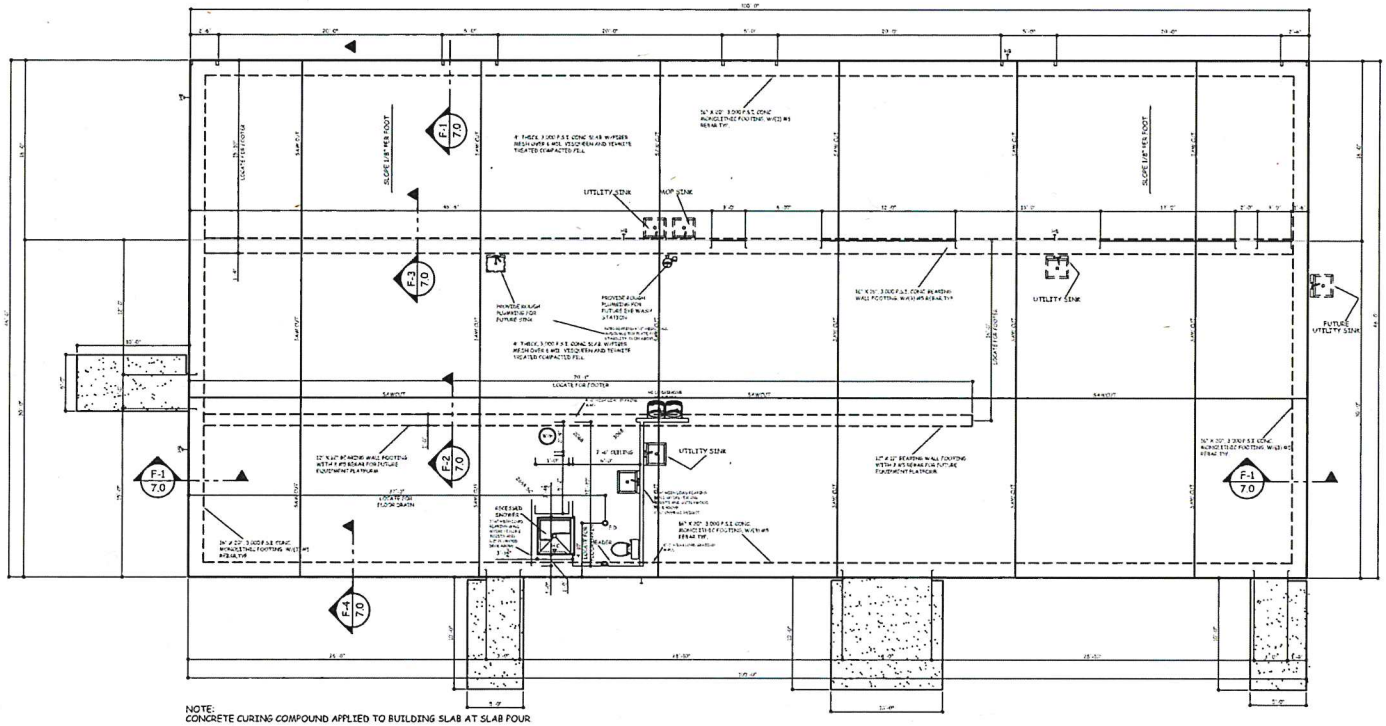
DETAIL F-1
16"X 20"
CONCRETE MONOLITHIC FOOTING
BASE RAIL ANCHORAGE



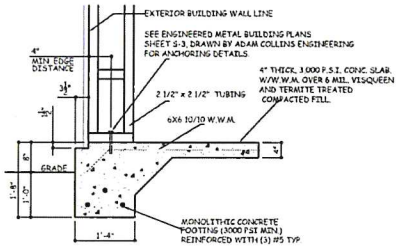
DETAIL F-2
12" X 12"
CONCRETE BEARING WALL FOOTING



DETAIL F-3
16" X 16"
CONCRETE BEARING WALL FOOTING



NOTE:
CONCRETE CURING COMPOUND APPLIED TO BUILDING SLAB AT SLAB POUR



DETAIL F-4
16"X 20"
CONCRETE MONOLITHIC FOOTING
BASE RAIL ANCHORAGE

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DRAWING # 24-5618

DATE:	4-23-23
REV:	12-14-23
REV:	12-21-23
REV:	12-22-23
REV:	12-28-23

CURINGTON Construction LLC
Office (352) 732-7839 Fax (352) 351-5044
2602 NE 24th Street Ocala, Florida 34470
CGC 1511120

H.W. Barrineau & Associates, Inc.
Civil Structural Engineering

Building Classification: Pre-Engineered Metal
Location: Parcel I.D.# 4036-1401-00

Drawn By: Luis Martical
Checked By: Don Curington

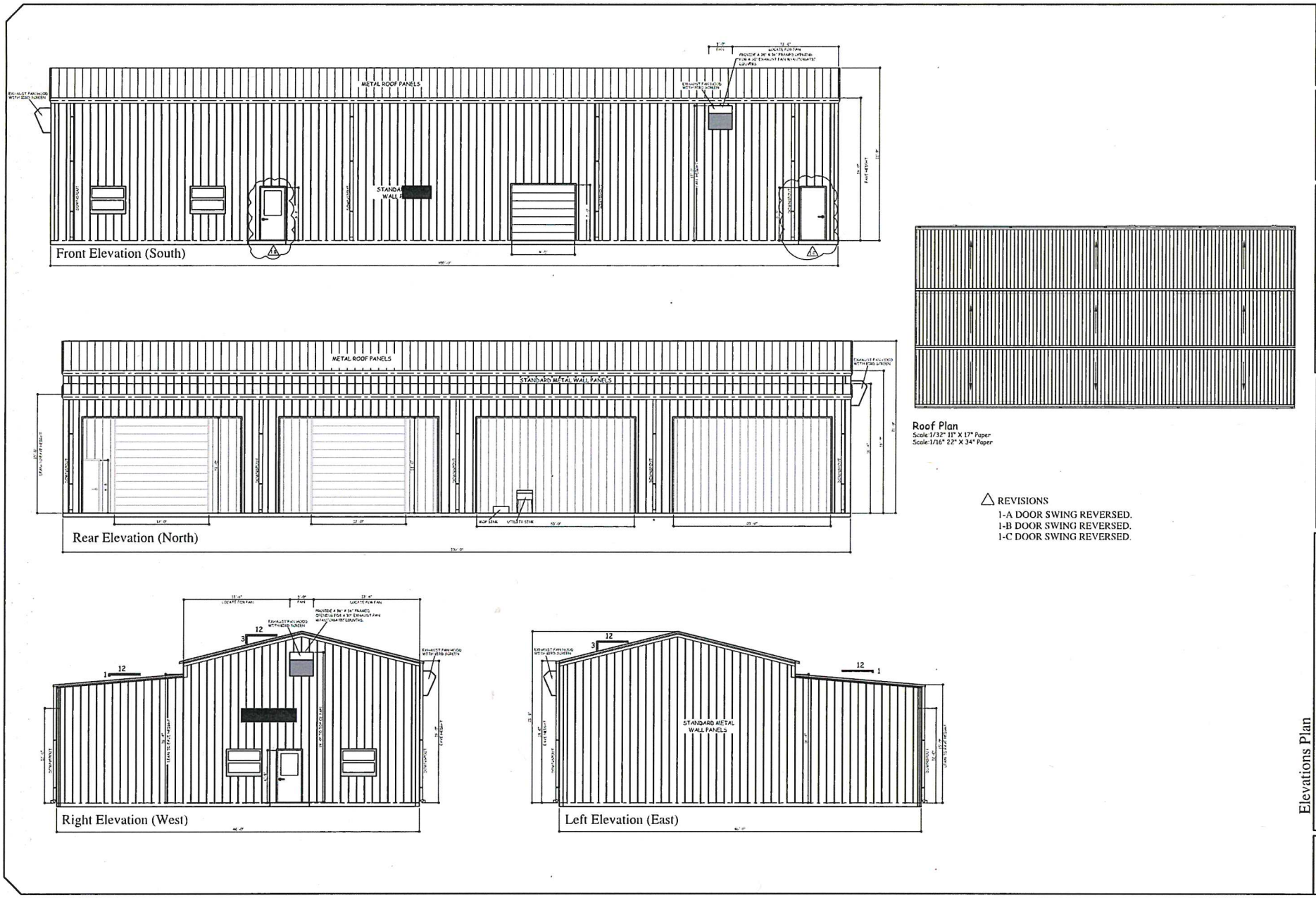
Scale: 3/8"=1'-0" On 22" X 34" Paper
3/32"=1'-0" On 11" x 17" Paper

Foundation Plan

FOR CONSTRUCTION

SHEET # 7.0

12/28/2024 5:19:11 AM



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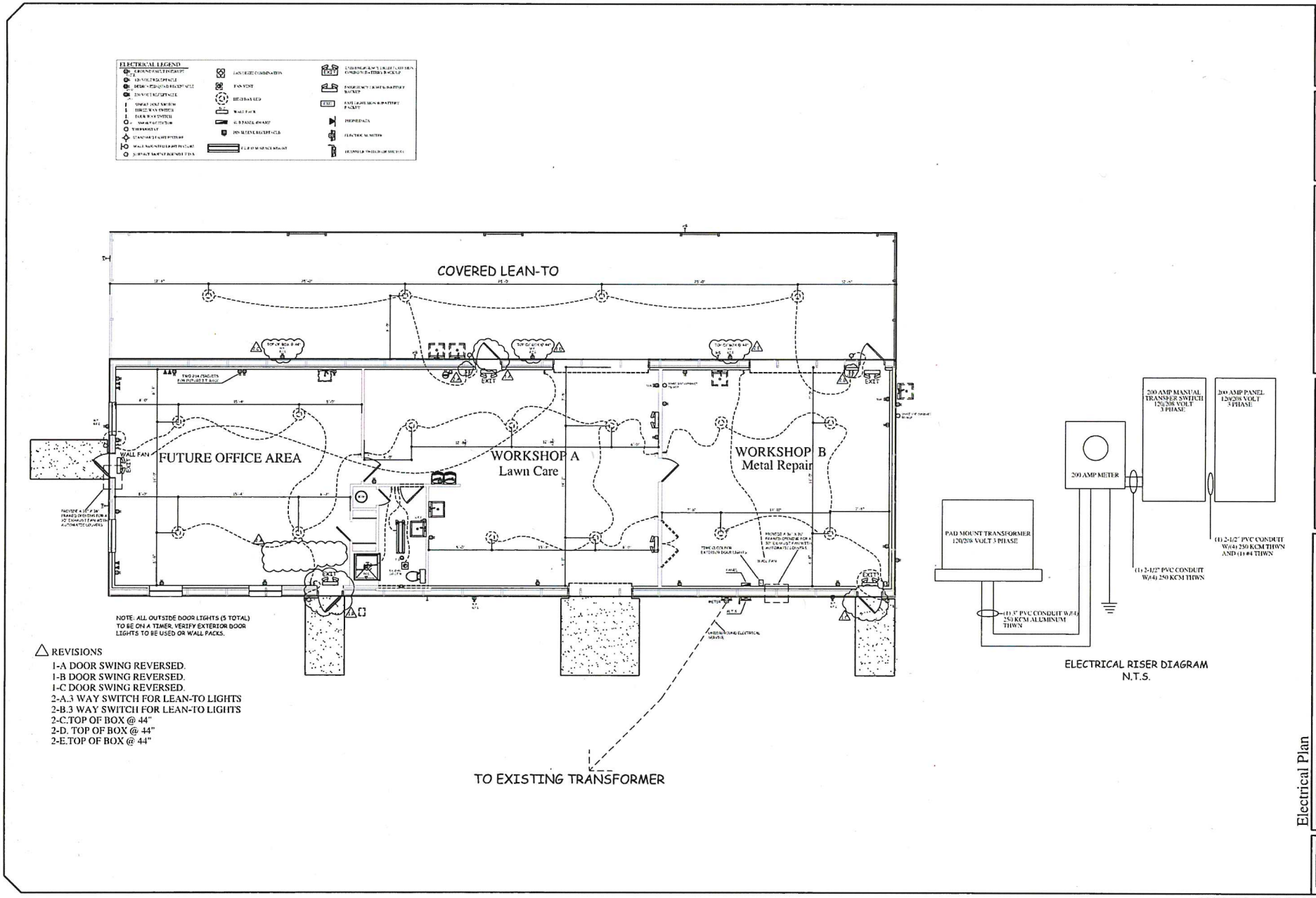
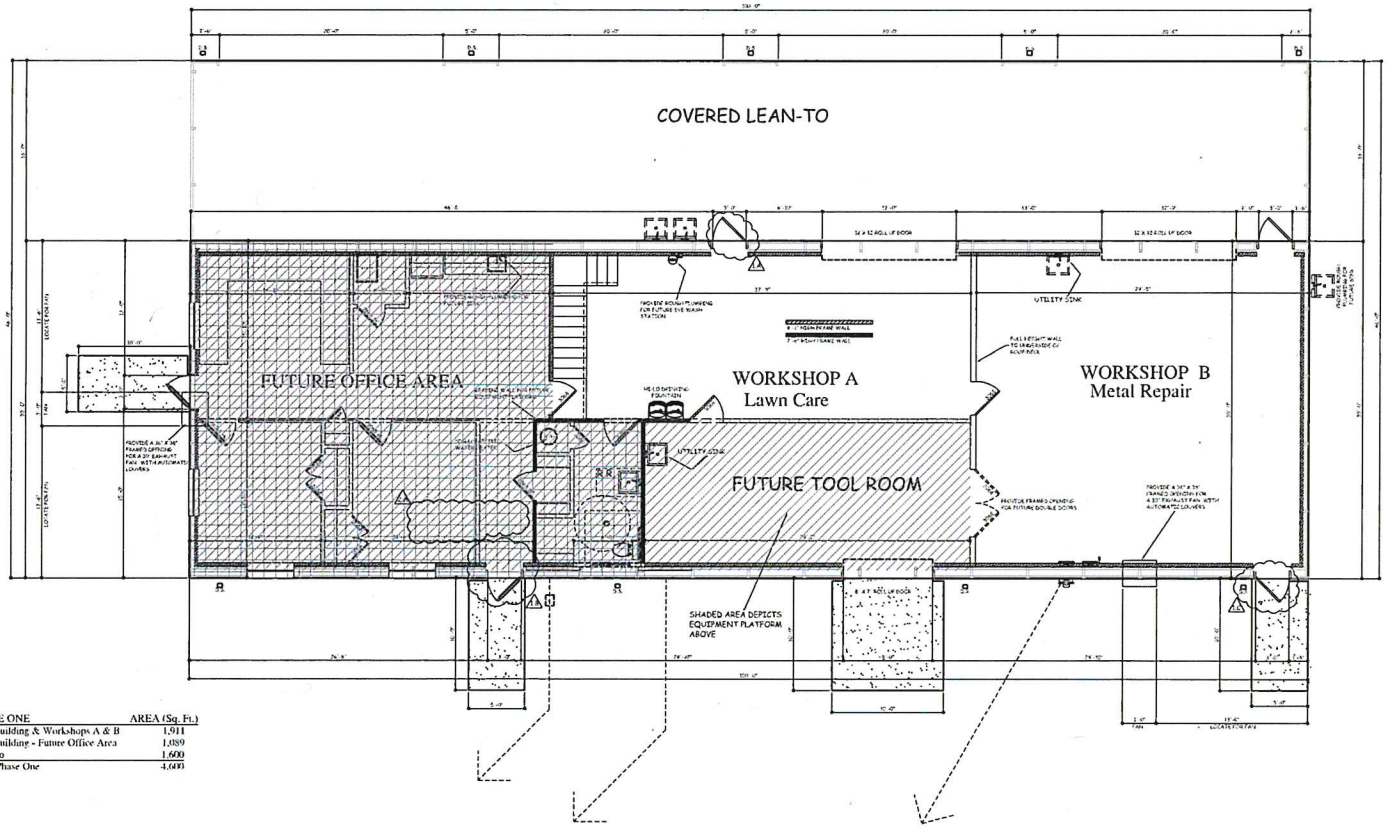

ELECTRICAL LEGEND
COVERED LEAN-TO
FUTURE OFFICE AREA
WORKSHOP A
Lawn Care
WORKSHOP B
Metal Repair
WALL FAN
EXIT
NOTE: ALL OUTSIDE DOOR LIGHTS (5 TOTAL) TO BE ON A TIMER. VERIFY EXTERIOR DOOR LIGHTS TO BE USED OR WALL PACKS.
REVISIONS
1-A DOOR SWING REVERSED.
1-B DOOR SWING REVERSED.
1-C DOOR SWING REVERSED.
2-A.3 WAY SWITCH FOR LEAN-TO LIGHTS
2-B.3 WAY SWITCH FOR LEAN-TO LIGHTS
2-C.TOP OF BOX @ 44"
2-D. TOP OF BOX @ 44"
2-E.TOP OF BOX @ 44"
TO EXISTING TRANSFORMER
200 AMP MANUAL TRANSFER SWITCH 120/208 VOLT 3 PHASE
200 AMP PANEL 120/208 VOLT 3 PHASE
200 AMP METER
PAD MOUNT TRANSFORMER 120/208 VOLT 3 PHASE
(1) 2-1/2" PVC CONDUIT W/(4) 250 KCM THWN AND (1) #4 THWN
(1) 2-1/2" PVC CONDUIT W/(4) 250 KCM THWN
(1) 3" PVC CONDUIT W/(4) 250 KCM ALUMINUM THWN
ELECTRICAL RISER DIAGRAM
N.T.S.
FOR CONSTRUCTION
24-5618
CURINGTON Construction LLC
Office (352) 732-7839 Fax (352) 351-5044
26052 N.E. 24th Street Ocala, Florida 34470
H.W. Barrineau & Associates, Inc.
Civil Structural Engineering
Electrical Plan
Youngfife Southwind Camp
Maintenance Building
Building Classification: Pre-Engineered Metal
Location:
Parcel I.D. # 40363-001-00
Scale:
1/16" = 1'-0" On 22" X 34" Paper
3/32" = 1'-0" On 11" x 17" Paper
Luis Mariscal
Checked By:
Dave Curington
SHEET #
E-100
12/28/2024 5:20:47 AM



PHASE ONE	AREA (Sq. Ft.)
Main Building & Workshops A & B	1,911
Main Building - Future Office Area	1,089
Lean-To	1,600
Total Phase One	4,600

Future Floor Plan

Youngllfe Southwind Camp
Maintenance Building
Drawn By: Luis Mariscal
Checked By: Dan Curington

Building Classification: Pre-Engineered Metal
Location: Parcel I.D.# 40263-001-00

Scale: 1/16"=1'-0" On 22" X 34" Paper
1/32"=1'-0" On 11" x 17" Paper

H.W. Barrineau & Associates, Inc.
Civil Structural Engineering

CURINGTON Construction LLC
Office (352) 732-7839 Fax (352) 351-5044
2652 N.E. 24th Street Ocala, Florida 34470
CGC 1531120

DATE:	4-15-21
REV:	12-10-21
REV:	12-21-21
REV:	12-22-21
REV:	12-20-24

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F-100